

Regression Models for the Prediction of Compressive Strength of Concrete with & without Fly ash

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Abstract- A mathematical analysis using statistical techniques for the prediction of compressive strength of concrete was performed for the concrete data obtained from laboratory experimental work done in this study. The variables used in the prediction models were the mix proportioning elements. The multiple non-linear regression models yielded excellent correlation coefficients for the prediction of compressive strength at different curing ages (28,56 & 91 days) as well as for other variations which include use of fly as a partial replacement for cement and variation in the zone of coarse aggregates.

Keywords- Mathematical model, Concrete, Compressive Strength, Multiple Regression, Statistical Analysis.

I. INTRODUCTION

Concrete is a wonder construction material which has changed the way construction is being carried out during the last century. Having the capability to be formed into any shape and size, it has transformed itself leading to development of many appealing structures. From a simple material easily formed by just adding coarse aggregates, sand, cement and water in desired proportions, it caught the fascination of researchers many decades ago. By playing around with its basic ingredients researchers have been able to develop concretes which not only have very high compressive strength, but have good durability properties as well. The results of compressive strengths vary not only for different concrete mixtures, but for the same mixture as well, which has been attributed to various factors (ACI214R-02).

Statistical procedures provide tools of considerable value when evaluating the results of strength tests. Information derived from such procedures is also valuable in refining design criteria and specifications. Statistical methods also have the added attraction that once fitted they can be used to perform predictions quickly and simpler to implement in software. Prediction of concrete strength, therefore, has been an active area of research and a considerable number of studies have been carried out. Many attempts have been made to obtain a suitable mathematical model which is capable of predicting strength of concrete at various ages with acceptable high accuracy.

The aim of the present study is to predict compressive strength of concrete for a given sample, as accurately as possible. For this purpose, multiple regression analysis is used for predicting the compressive strength of concrete using four variables, namely, water-cementations ratio, fine aggregate-cementitious ratio, coarse aggregate-cementitious ratio and cementitious content. Regression models have been developed for partial replacement of cement with fly ash (0 and 15 percent), zones of aggregates(A,B and C) and curing ages(28,56 and 91 days).

II. EXPERIMENTAL DATASET

Data for the present work has been taken from the experiments conducted by Kumar (2003). For generating a reliable data bank on concrete compressive strength, he has considered seven parameters,namely, water-

cementitious material ratio, cementitious content, water content, percentage partial replacement of cement by fly ash, workability, aggregate zones and curing ages in his experiments. The experiments were performed in controlled laboratory conditions. Table 1 shows the variations in the values of parameters as taken by Kumar (2003). As can be observed from the Table 1, coarse aggregates are divided into three zones. A set of 15 cubes for each of mixes so proportioned were cast and tested after 28, 56 and 91 days of curing. Thus, an extensive data bank for analyzing compressive strength of concrete has been generated and the same has been used in the present work.

Table 1 Range of values of various parameters

Water-cementitious ratio	0.42 – 0.55
Cementitious content	350 – 475 @ 25 kg/m ³
Water content	180-230 @ 10 kg/m ³
Percentage replacement of cement by fly ash	0 and 15 per cent
Workability	Medium and high
Coarse Aggregates, Zones	A, B and C
Curing ages, days	28, 56, 91

The physical properties of the materials used in the study are shown in Table 3. Ordinary Portland Cement (OPC) of 43 grade (as per IS:8112-1989) was used. It has a specific gravity of 3.12 and attained a compressive strength of 46.50MPa after 28 days of curing. The fine aggregates used had a specific gravity of 2.54 and belonged to zone – II of the grading zones as per IS:383-1970. Three types of coarse aggregates were used in varying proportions, splitting them into three zones as shown in Table 2. The 20mm coarse aggregates had a specific gravity of 2.61, the 10mm aggregates had a specific gravity of 2.63 and 4.75 aggregates had a specific gravity of 2.58. Only fly ash was used, as a mineral admixture, as a partial replacement of cement to study the effect on compressive strength of concrete. The physical properties of fly ash used are shown in Table 4.

As the aim of the study is studying the effect of mix proportions on the compressive strength of concrete, different mix proportions were used. The details of the mix proportions using different zones of coarse aggregates are shown in Tables 5 to 10. The compressive strength test was performed and evaluated in accordance with IS:519. Specimens were immersed in water until the day of testing at 28, 56 and 91 days.

Table 2: Zones of Aggregates

Zone	Percentage passing 20mm sieve and retained on 10mm sieve, CA-I	Percentage passing 10mm sieve and retained on 4.75mm sieve, CA-II	Percentage passing 4.75mm sieve and retained on 2.36 mm sieve, CA-III	Fineness modulus
A	67	33	-	6.67
B	50	50	-	6.50
C	-	50	50	5.50

Table 3: Physical Properties of Materials

Materials	Properties
Ordinary Portland Cement (OPC)	Grade: 43, as per IS:8112-1989

	Specific Gravity: 3.12 7 days compressive strength: 35.50 MPa 28 days compressive strength: 46.50 MPa
Fine aggregates (FA)	Zone: III Fineness modulus: 2.09 Specific Gravity: 2.54
Coarse Aggregates – I (CA-I)	Specific Gravity: 2.61
Coarse Aggregates – I (CA-II)	Specific Gravity: 2.63
Coarse Aggregates – I (CA-III)	Specific Gravity: 2.58

Table 4 Physical properties of fly ash

S. No.	Property	Observed values
1.	Specific gravity	1.99
2.	Lime reactivity – average compressive strength at Blaine 3389 cm ² /gm	47.87 kg/cm ²
3.	Colour	Grey (blackish)
4.	Fineness modulus	0.68

III. MODELING OF DATASET AND METHOD

In this study, the mean value of compressive strength of concrete at 28,56 and 91 days are predicted and the output of one is used as one of the input variable for the prediction of another like the predicted compressive strength of 28 days(output of 28 days) is entered as one of the input variable to predict the compressive strength of 56 days and the output of 28 days and 56 days are accommodated as input variables to predict the compressive strength of 91 days. Other than this, 91 days compressive strength is also predicted using only strength of 28 days and also using only strength of 56 days. The final form of the regression equations for different cases is provided as below:

$$f_{c28} = A_0 (W/CM)^{A_1} (FA/CM)^{A_2} (CA/CM)^{A_3} \dots\dots\dots(1)$$

$$f_{c56} = A_0 (W/CM)^{A_1} (FA/CM)^{A_2} (CA/CM)^{A_3} (f_{c28}/CM)^{A_4} \dots\dots\dots(2)$$

$$f_{c91} = A_0 (W/CM)^{A_1} (FA/CM)^{A_2} (CA/CM)^{A_3} (f_{c28}/CM)^{A_4} (f_{c56}/CM)^{A_5} \dots\dots\dots(3)$$

$$f_{c91,28} = A_0 (W/CM)^{A_1} (FA/CM)^{A_2} (CA/CM)^{A_3} (f_{c28}/CM)^{A_4} \dots\dots\dots(4)$$

$$f_{c91,56} = A_0 (W/CM)^{A_1} (FA/CM)^{A_2} (CA/CM)^{A_3} (f_{c56}/CM)^{A_4} \dots\dots\dots(5)$$

where, f_{c28} is the compressive strength of concrete after 28 days of curing, f_{c56} is the compressive strength of concrete after 56 days of curing, f_{c91} is the compressive strength of concrete after 91 days of curing. In the equations from (1) to (5), in predicting the strength for higher ages, the strength of concrete at lower ages has also been considered in the model developed. The regression coefficients so obtained for each of the cases is provided in Tables 5 to 10.

Table5. Regression coefficients of multiple regression models predicting compressive strength of Concrete with Zone A of aggregates without any replacement of cement with fly ash

Without replacement of cement by fly ash					
	f_{c28}	f_{c56}	f_{c91}	$f_{c91,28}$	$f_{c91,56}$
A₀	3.1688	15.0896	26.3395	14.1329	30.8196
A₁	-3.1459	-2.6449	-1.5201	-1.8367	-1.5043
A₂	0.7646	0.8674	0.2932	0.3850	0.3429
A₃	0.1286	0.0058	-0.1769	-0.0292	-0.1599
A₄		0.4406	-0.4689	0.0481	
A₅			0.6463		0.2355
Correlation Coefficient	0.9416	0.9484	0.9711	0.9534	0.9609

Table6. Regression coefficients of multiple regression models predicting compressive strength of Concrete with Zone A of aggregates with 15% replacement of cement with fly ash

With 15% replacement of cement by fly ash					
	f_{c28}	f_{c56}	f_{c91}	$f_{c91,28}$	$f_{c91,56}$
A₀	8.4570	1.8925	17.8367	18.4700	18.7979
A₁	-2.1654	-2.4073	-1.0491	-1.0127	-1.0562
A₂	0.2990	0.0837	-0.0544	-0.0716	-0.0355
A₃	-0.2019	0.2156	-0.0703	-0.1155	-0.0669
A₄		-0.4584	-0.0341	-0.1510	
A₅			-0.1067		-0.1140
Correlation Coefficient	0.9670	0.9289	0.9842	0.9805	0.9841

Table7. Regression coefficients of multiple regression models predicting compressive strength of Concrete with Zone B of aggregates without any replacement of cement with fly ash

Without replacement of cement by fly ash					
	f_{c28}	f_{c56}	f_{c91}	$f_{c91,28}$	$f_{c91,56}$
A₀	4.9848	9.4214	2.9227	2.9093	3.7531
A₁	-2.6623	-2.0157	-2.6871	-2.6907	-2.5446
A₂	0.5947	0.4259	0.5915	0.5923	0.5726
A₃	0.1152	0.1029	0.3159	0.3161	0.2954
A₄		-0.0037	-0.2122	-0.2176	
A₅			-0.0050		-0.1716
Correlation Coefficient	0.9756	0.9333	0.9748	0.9748	0.9743

Table8. Regression coefficients of multiple regression models predicting compressive strength of Concrete with Zone B of aggregates with 15% replacement of cement with fly ash

With 15% replacement of cement by fly ash					
	f_{c28}	f_{c56}	f_{c91}	$f_{c91,28}$	$f_{c91,56}$
A₀	10.4992	3.5841	20.3021	23.0852	18.0747
A₁	-1.8512	-2.8598	-1.1693	-0.9357	-1.0801
A₂	0.1232	0.6016	0.0316	-0.1184	-0.0796
A₃	-0.1086	0.3275	-0.0189	-0.1252	-0.0667
A₄		0.0263	0.2148	-0.0935	
A₅			-0.2478		-0.1308
Correlation Coefficient	0.9885	0.9471	0.9946	0.9874	0.9922

Table9. Regression coefficients of multiple regression models predicting compressive strength of Concrete with Zone C of aggregates without any replacement of cement with fly ash

Without replacement of cement by fly ash					
	f_{c28}	f_{c56}	f_{c91}	$f_{c91,28}$	$f_{c91,56}$
A₀	0.2970	4.152	1.5065	2.4590	2.1313
A₁	-6.0311	-3.4970	-4.7240	-4.1082	-4.3820
A₂	2.3580	1.3572	1.8668	1.5940	1.7299
A₃	0.1728	-0.0013	0.1001	0.0914	0.0786
A₄		0.1490	-0.156	0.1438	
A₅			0.3275		0.1874
Correlation Coefficient	0.9784	0.9581	0.9893	0.9876	0.9889

Table10. Regression coefficients of multiple regression models predicting compressive strength of Concrete with Zone C of aggregates with 15% replacement of cement with fly ash

With 15% replacement of cement by fly ash					
	f_{c28}	f_{c56}	f_{c91}	$f_{c91,28}$	$f_{c91,56}$
A₀	154.1932	6.6923	19.5303	23.1378	17.4536
A₁	1.3580	-1.8834	-0.9190	-0.7240	-1.1048
A₂	-1.4933	-0.0180	-0.2415	-0.3368	-1.1340
A₃	-0.1184	0.0518	0.0397	0.0328	0.0375
A₄		-0.1823	-0.0590	-0.1117	
A₅			-0.0559		-0.0986
Correlation Coefficient	0.9945	0.9636	0.9780	0.9579	0.9680

IV.CONCLUSION

Table 5, Table 7, and Table 9 have the regression coefficients of multiple regression models predicting compressive strength of concrete with zone A, zone B and zone C of aggregates respectively without any replacement of cement with fly ash. Table 6, Table 8, and Table 10 have the regression coefficients of multiple regression models predicting compressive strength of concrete with zone A, zone B and zone C of aggregates respectively with 15% replacement of cement with fly ash. In Table 5 and Table 6 ,during the curing days, f_{c28} , f_{c91} , $f_{c91,28}$ and $f_{c91,56}$

correlation coefficients of with fly ash are 0.9670, 0.9842, 0.9805 and 0.9841 respectively, are better than without fly ash. In f_{c56} only correlation coefficient is 0.9289, when replaced with fly ash. In Table 7 and Table 8, during the curing days, f_{c28} , f_{c91} , $f_{c91,28}$ and $f_{c91,56}$ correlation coefficients of with fly ash are 0.9885, 0.9946, 0.9874 and 0.9922, respectively, are better than without fly ash. In the curing day of f_{c56} , when replaced with fly ash the correlation coefficient is 0.9471, but it is still better than without replacement of fly ash. In Table 9 and Table 10 is with the results of zone C. Zone C aggregates have better correlation coefficients when it is without replacement of fly ash. During the curing days of f_{c28} , f_{c56} , f_{c91} , $f_{c91,28}$ and $f_{c91,56}$; correlation coefficients of without fly ash are 0.9784, 0.9581, 0.9893, 0.9876 and 0.9889 respectively and correlation coefficients with fly ash are 0.9945, 0.9636, 0.9780, 0.9579 and 0.9680 respectively. Therefore, it has been seen that zone A and zone B aggregates are giving better correlation coefficients, when replaced cement 15% with fly ash; and zone C aggregates are working better without fly ash.

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